

# **APPLICATION FOR** HISTORIC AREA WORK PERMIT HISTORIC PRESERVATION COMMISSION 301.563.3400

HAWP#\_\_ DATE ASSIGNED\_\_\_\_

FOR STAFF ONLY:

#### **APPLICANT:**

Name:			E-mail:		
Address:			City:	Zip:	_
Daytime Phor	ne:		Tax Account	No.:	
AGENT/CON	TACT (if applicable	e):			
Name:			E-mail:		
Address:			City:	Zip:	
Daytime Phor	ne:		Contractor R	egistration No.:	
LOCATION O	F BUILDING/PREN	IISE: MIHP # of Histor	ic Property		
map of the ea Are other Plan (Conditional L supplemental Building Num	asement, and docu nning and/or Heari Jse, Variance, Reco I information.	mentation from the Eang Examiner Approvals rd Plat, etc.?) If YES, in	sement Holde  A / Reviews Red  A clude informa	et on the Property? If YES, includer supporting this application.  Quired as part of this Application tion on these reviews as	1? 
		Subdivision:			
for propose be accepted     New Co     Addition     Demol     Gradin I hereby cert and accurate	d work are submit for review. Check onstruction ition g/Excavation ify that I have the are and that the construction if the construction is and that the construction is the construction if the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction in the construction is the construction in the construction	tted with this applicated and that apply:  Deck/Porch Fence Hardscape/Lands Roof Buthority to make the foruction will comply wi	ation. Incomp scape oregoing appli th plans review be a condition	fy that all supporting items lete Applications will not Shed/Garage/Accessory Struct Solar Tree removal/planting Window/Door Other: cation, that the application is colved and approved by all necessary for the issuance of this permit.	orrect

# HAWP APPLICATION: MAILING ADDRESSES FOR NOTIFING [Owner, Owner's Agent, Adjacent and Confronting Property Owners] Owner's mailing address Owner's Agent's mailing address Adjacent and confronting Property Owners mailing addresses

Description of Property: Please describe the building and surrounding environment. Include information on significant structures, landscape features, or other significant features of the property:
Description of Work Proposed: Please give an overview of the work to be undertaken:

Work Item 1:	
Description of Current Condition:	Proposed Work:
Work Item 2:	
Description of Current Condition:	Proposed Work:
Work Item 3:	
Description of Current Condition:	Proposed Work:

# HISTORIC AREA WORK PERMIT CHECKLIST OF APPLICATION REQUIREMENTS

	Required Attachments						
Proposed Work	I. Written Description	2. Site Plan	3. Plans/ Elevations	4. Material Specifications	5. Photographs	6. Tree Survey	7. Property Owner Addresses
New Construction	*	*	*	*	*	*	*
Additions/ Alterations	*	*	*	*	*	*	*
Demolition	*	*	*		*		*
Deck/Porch	*	*	*	*	*	*	*
Fence/Wall	*	*	*	*	*	*	*
Driveway/ Parking Area	*	*		*	*	*	*
Grading/Exc avation/Land scaing	*	*		*	*	*	*
Tree Removal	*	*		*	*	*	*
Siding/ Roof Changes	*	*	*	*	*		*
Window/ Door Changes	*	*	*	*	*		*
Masonry Repair/ Repoint	*	*	*	*	*		*
Signs	*	*	*	*	*		*



Photo 1: Existing East side of the house- Showing porch doors to be removed and second floor bedroom to be renovated

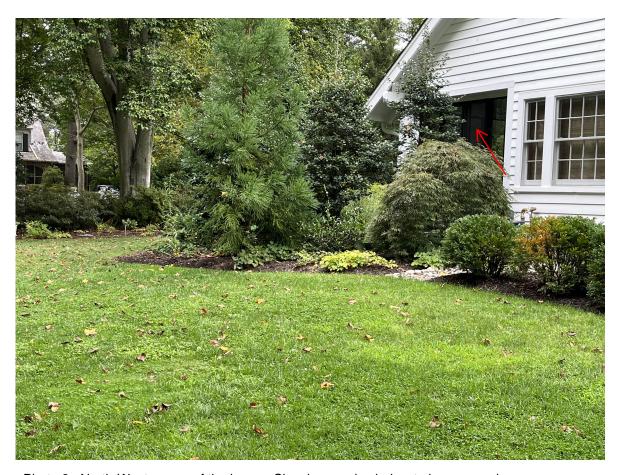


Photo 2: North-West corner of the house. Showing porch window to be removed



Photo 3: Showing East side of the house- Porch doors to be removed



Photo 4: East side house. Showing evergreen tree to be removed.



Photo 5: Showing evergreen tree to be removed



Photo 5: East side house. Showing preferred outdoor seating area.



# **Municipality Letter for Proposed Construction Project**

**Subject Property:** 20 West Kirke Street, Chevy Chase, MD 20815

**Property Owner:** Elizabeth Williams

Project Manager/Contractor: Avantika Dalal /Moody Graham Landscape Architecture

**Proposed Work:** Restoration/reconstruction of side porch and interior renovations

to second floor bedroom suites, including bathrooms

3/4/2025

Rabbiah Sabbakhan, Director
Department of Permitting Services of Montgomery County
255 Rockville Pike, 2<sup>nd</sup> floor
Rockville, MD 20850

Dear Mr. Sabbakhan,

This letter is to inform your department that the above homeowner/contractor has notified Chevy Chase Village that he or she plans to apply for both county and municipal permits for the above summarized construction project. Chevy Chase Village will not issue any municipal building permit(s) for this proposed project until Montgomery County has issued all necessary county permits and the applicant has provided Chevy Chase Village with copies of county-approved and stamped plans. We have advised the homeowner/contractor that a permit from Montgomery County does not guarantee a permit from this municipality unless the project complies with all our municipal rules and regulations.

If this homeowner/contractor later applies for an amended county permit, please do not approve that application until you have received a Municipality Letter from us indicating that the homeowner/contractor has notified us of that proposed amendment to the permit.

If you have any questions about this proposed project and the municipal regulation of it by Chevy Chase Village, do not hesitate to have your staff contact my office. The Village Permitting Coordinator can be reached by phone at 301-654-7300 or by e-mail at covpermitting@montgomerycountymd.gov.

Sincerely,

Shana R. Davis-Cook

Chevy Chase Village Manager

CHEVY CHASE VILLAGE

5906 Connecticut Avenue Chevy Chase, Maryland 20815

Phone (301) 654-7300 Fax (301) 907-9721

ccv@montgomerycountymd.gov www.chevychasevillagemd.gov **BOARD OF MANAGERS** 

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LOU MORSBERGER

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VILLAGE MANAGER SHANA R. DAVIS-COOK LEGAL COUNSEL SUELLEN M. FERGUSON

# WILLIAMS

20 WEST KIRKE STREET CHEVY CHASE, MD 20815

# ADDITIONS, ALTERATIONS AND REPAIRS ON **EXISTING STRUCTURES** SHALL COMPLY WITH IRC 2018 SECTION R-102.7.1

# RESIDENCE

# GENERAL DATA

# **GENERAL DATA:**

20 W KIRKE STREET, ADDRESS:

TOWN OF CHEVY CHASE, MONTGOMERY COUNTY, MARYLAND

PARCEL ID: SUBDIVISION: 0009 / BLOCK: 32 / LOT: PI3

ZONING DISTRICT: R-60

RESTORATION/RECONSTRUCTION OF SIDE PORCH AND PROJECT SCOPE: INTERIOR RENOVATIONS TO (2) SECOND FLOOR BEDROOM

SUITES, INCLUDING BATHROOMS

NO. DWELLING UNITS: 1 (N.C.)

2 PLUS BASEMENT (N.C.) NO. OF STORIES:

BUILDING HEIGHT: E.T.R. (N.C.)

5,638 SF ABOVE GRADE (N.C.) BUILDING AREA (UA):

LOT AREA: 18.136 S.F. LOT COVERAGE 4,743 S.F

WORK AREA (WA): 1,115 S.F.

# BUILDING CODES

EXECUTIVE REGULATION 31-19 2018 INTERNATIONAL RESIDENTIAL CODE (IRC)

2018 INTERNATION EXISTING BUILDING CODE (IEBC) 2018 INTERNATIONAL ENERGY CONSERVATION CODE - RESIDENTIAL PROVISIONS (IECC)

<u>STRUCTURAL ENGINEER:</u>

COBB ARCH. ENGINEERS LLC

OCCUPANCY USE GROUP: R-3

CONSTRUCTION TYPE: VB

# PROJECT TEAM

SDK ARCHITECTURE INC SYDNEY D KATZ 427 BOYD AVENUE TAKOMA PARK, MD 310-467-5907

SDK@SDK-ARCH.COM

CHRIS COBB 210 N LEE STREET ALEXANDRIA, VA 22314 703-350-4151 CCOBB@COBBAE.COM

(215) 680-5953

NADER KALHOR 1427 BOLTON ST., STE BALTIMORE, MD 21217 443-850-6885 NADER@APEXBUILDERSGRP.COM

**STRUCTURAL SHEETS:** 

GENERAL NOTES

DETAILS

LEGEND AND SCHEDULES

ROOF FRAMING PLAN

FIRST FLOOR FRAMING PLAN

SECOND FLOOR FRAMING PLAN

APEX BUILDERS GROUP

<u>INTERIOR DESIGNER:</u> WILLIAMS MONA ROSS BERMAN INTERIORS 20 WEST KIRKE STREET 3747 RIDGE AVE CHEVY CHASE, MD 20815 PHILADELPHIA, PA 19132 MONA R. BERMAN

# FULL DRAWING INDEX

### ARCHITECTURAL SHEETS:

COVER SHEET SITE PLAN

DEMOLITION PLAN - LOWER LEVEL DEMOLITION PLAN - FIRST FLOOR DEMOLITION PLAN - SECOND FLOOR

PROPOSED PLAN - LOWER LEVEL PROPOSED PLAN - FIRST FLOOR PROPOSED PLAN - SECOND FLOOR

BUILDING SECTION

EXTERIOR ELEVATIONS

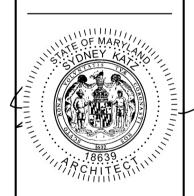
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427 Boyd Avenue

Takoma Park, MD 20912

Sydney Kat

2025.03.0



GENERAL NOTES

ALT

ALUM

APPL

ACT

BLDG

BLKG

BOT

BRG BTWN

CFM CJ

ČLG

CLR

CMU

COL

CONC

CONST

CORR

CTR/C.L.

CPT CT

DBL

DH

DIA

DIM

DN DR

DS

EJ

ELEC

ELEV

ENCL

EQUIP

EW

EXT

FDN

**FLUOR** 

F.O.

FRM

GAL

GFCI

GYP

J-BOX

EXIST EXP

DWG

CONT

**APPROX** 

ACOUSTICAL CEILING TILE

CUBIC FEET PER MINUTE

CONCRETE MASONRY UNIT

CONTROL JOINT

CLEAR(ANCE)

CONCRETE

CONTINUOUS

CORRIDOR

CERAMIC TILE

DOUBLE HUNG

DIAMETER

DIMENSION

DOWNSPOUT

DRAWING

ELEVATION

**ENCLOSURE** 

FACH WAY

EXPANSION

FINISH

FRAME

GALLON

GYPSUM

FLUORESCEN<sup>-</sup>

FOOT(FEET)

GALVANIZED

**GENERATOR** 

GENERAL CONTRACTOR

GROUND FAULT CIRCUIT

INTERRUPTER

GYPSUM WALLBOARD

GLASS, GLAZING

HOLLOW CORE

HOLLOW METAL

HEATING, VENTILATING \$

HOT WATER (HEATER)

INSIDE DIAMETER

INS/INSUL INSULATED/INSULATION

JUNCTION BOX

INTERIOR

KILOWATT

AIR-CONDITIONING

HORIZONTAL

ELECTRIC(AL

EXPANSION JOINT

DOOR

DEMOLISH, DEMOLITION

CARPET

CONSTRUCTION

COMMUNICATION(S)

BUILDING

BLOCKING

BEARING

BETWEEN

MASONRY

MECHANICAL

MOUNTED

MEZZANINE

MICROWAVE

NOT IN CONTRACT

ON CENTER(S)

OPPOSITE HAND (REVERSED)

POUNDS PER CUBIC FOOT

POUNDS PER SQUARE INCH

REFLECTED CEILING PLAN

PRESSURE TREATED

PLASTIC LAMINATE

MULLION

NUMBER

NOMINAL

**OPENING** 

OPPOSITE

**PLYWOOD** 

POLISHED

POWER

QUANTITY

RADIUS

RECEPTACLE

**REFRIGERATOR** 

REINFORCE(ING)

SMOKE DETECTOR

SOUND TRANSMISSION

COEFFICIENT

STRUCTURAL (ENGINEER)

SPECIFICATION

STORAGE

SECTION

SYMMETRICAL

STAINLESS STEEL

TEMPERATURE THICK(NESS) TO MATCH EXISTING

TOP OF

TYPICAL

TREAD(S)

VESTIBULE

VINYL TILE

MOOD

WEIGHT MITH

WITHOUT

YARD

TONGUE AND GROOVE

UNDERWRITERS' LABORATORY

UNLESS NOTED OTHERWISE

VINYL COMPOSITION TILE

VAPOUR BARRIER

VERIFY IN FIELD

WELDED WIRE FABRIC

REVISION(S), REVISED

METAL MECHANICAL

MISCELLANEOUS

MASONRY OPENING

MAX

MECH

MEMB

MISC

MO

MTL

MEZZ

NO OR #

NOM

POL

QTY

REV

STOR

TEMP

VEST

YD

- GENERAL CONTRACTOR SHALL NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCY IN THE DOCUMENTS OR EXISTING CONDITIONS. WORK THAT PROCEEDS WITHOUT NOTIFYING THE ARCHITECT IS AT THE CONTRACTOR'S OWN RISK.
- 2. BEFORE COMMENCEMENT OF ANY WORK THAT CHANGES THE CONTRACT SUM OR CONTRACT TIME, WRITTEN AUTHORIZATION MUST BE OBTAINED FROM THE ARCHITECT. WORK THAT PROCEEDS WITHOUT WRITTEN AUTHORIZATION FROM THE ARCHITECT IS AT THE CONTRACTOR'S OWN RISK
- 3. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL WORK,
  - THIS INCLUDES BUT IS NOT LIMITED TO:
  - a. PRE-BID SITE VISIT FOR VERIFICATION OF EXISTING CONDITIONS.
  - b. FIELD DIMENSIONS AS REQUIRED

d. ALL MEANS AND METHODS

c. CONCEALMENT OF MECHANICAL/ELECTRICAL SERVICES BEHIND BUILDING FINISHES UNLESS NOTED OTHERWISE.

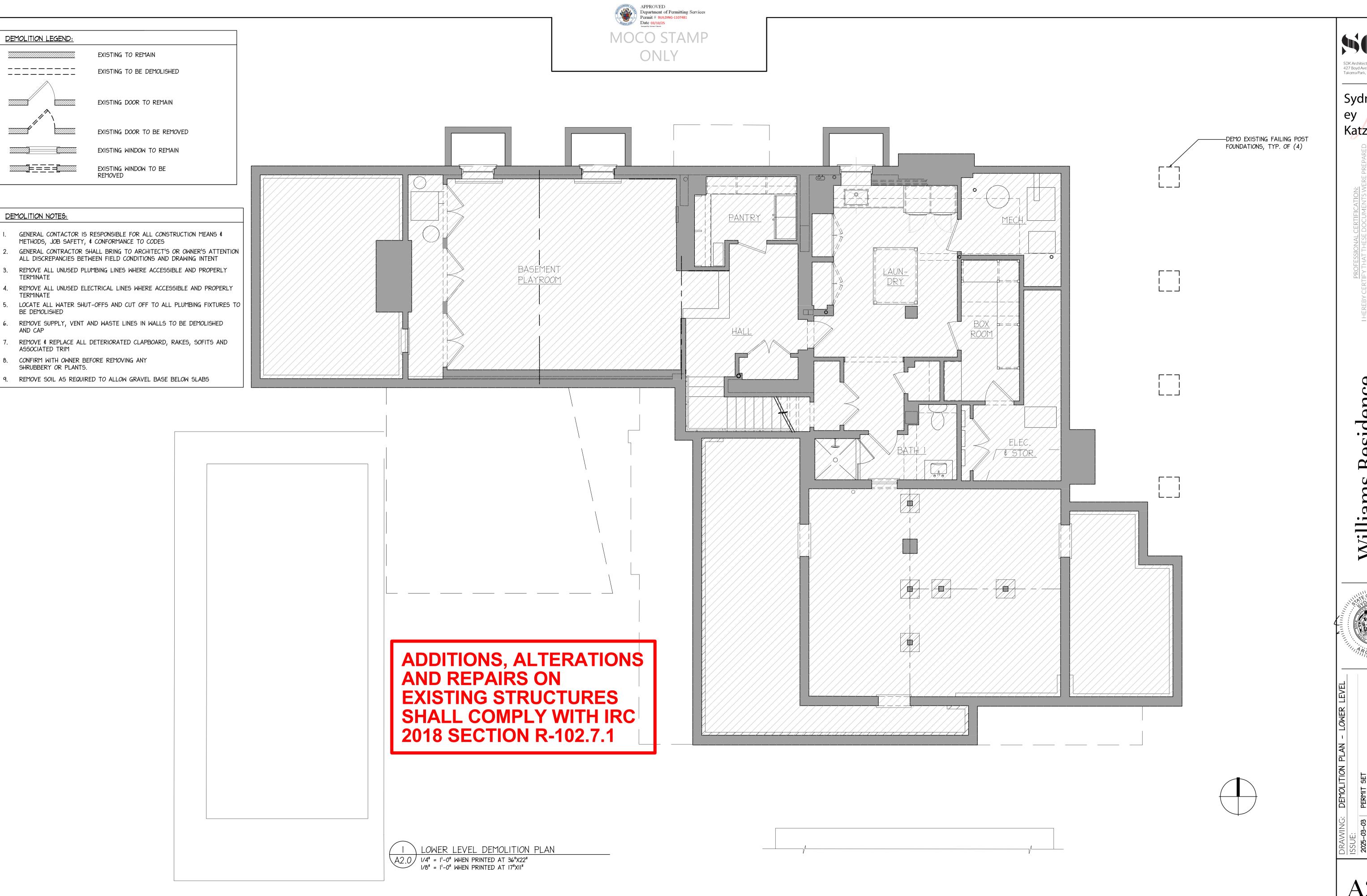
4. CONSTRUCTION SHALL CONFORM TO ALL CODES AND REGULATIONS HAVING JURISDICTION FOR THIS

- 5. THE MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS SHALL BE UPDATED AS REQUIRED.
- GENERAL CONTRACTOR SHALL PROVIDE PROPOSALS AND SHOP DRAWINGS FOR REVIEW AND APPROVAL BY ARCHITECT AND OWNER
- 6. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL NECESSARY PERMITS ARE OBTAINED PRIOR TO PROCEEDING. WORK THAT PROCEEDS WITHOUT A PERMIT IS AT THE CONTRACTOR'S RISK.

SDK Architecture Inc 427 Boyd Avenue Takoma Park, MD 20912

Sydn Digitally signed by Sydney Katz ey Date: 2025.03.03
Katz 13:04:36 -05'00'

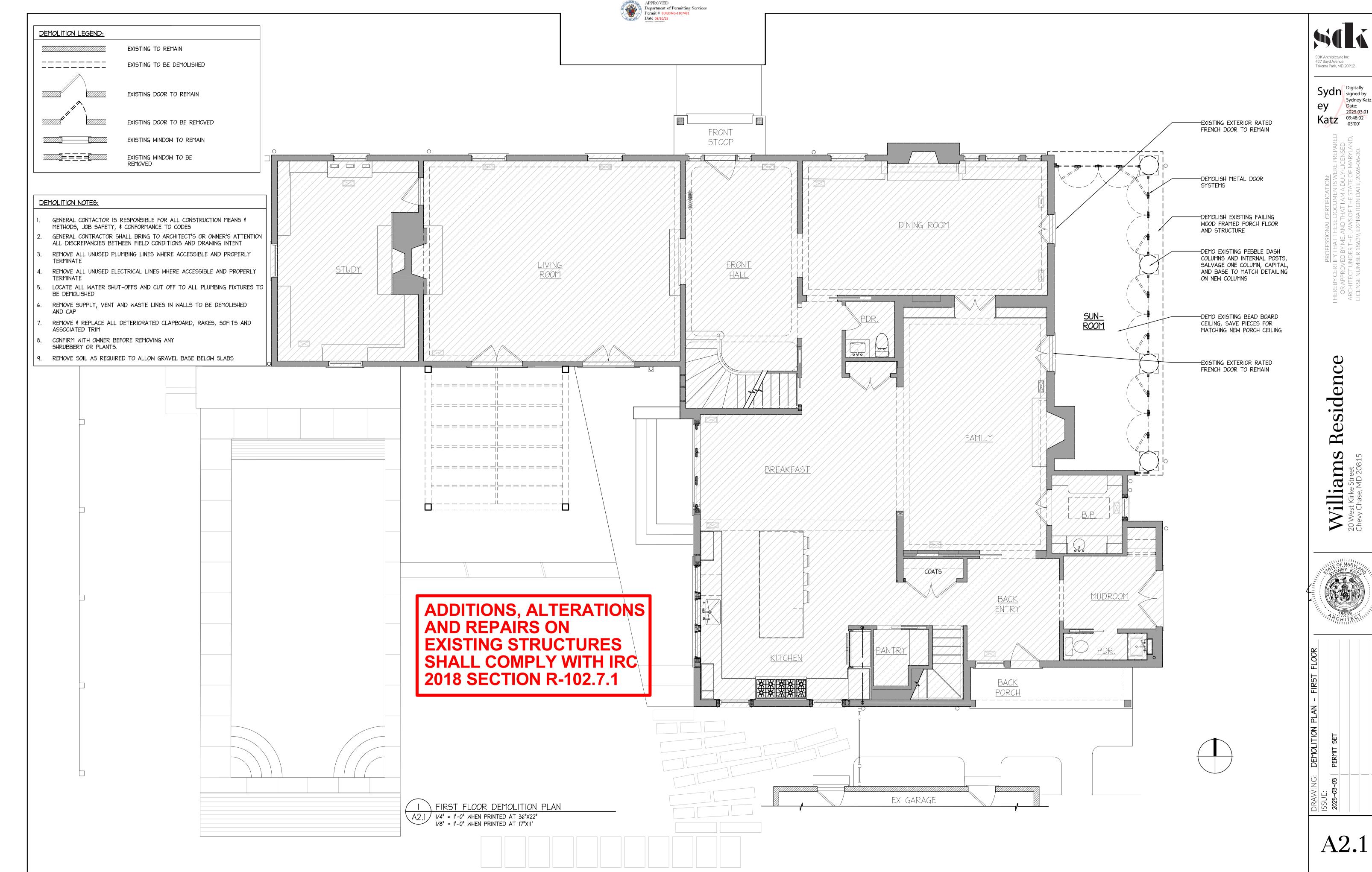
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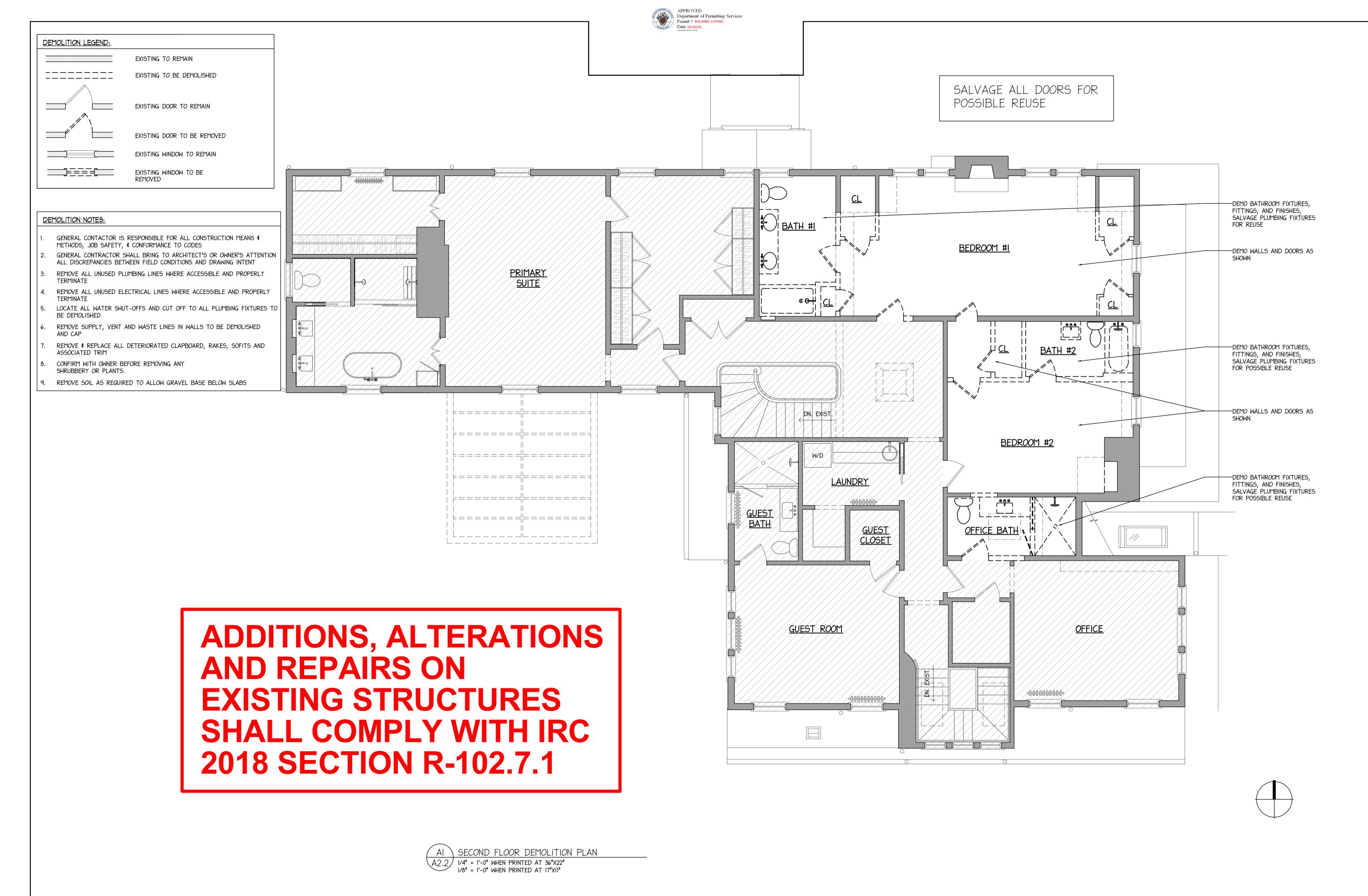
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A2.0

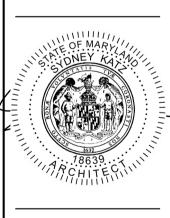




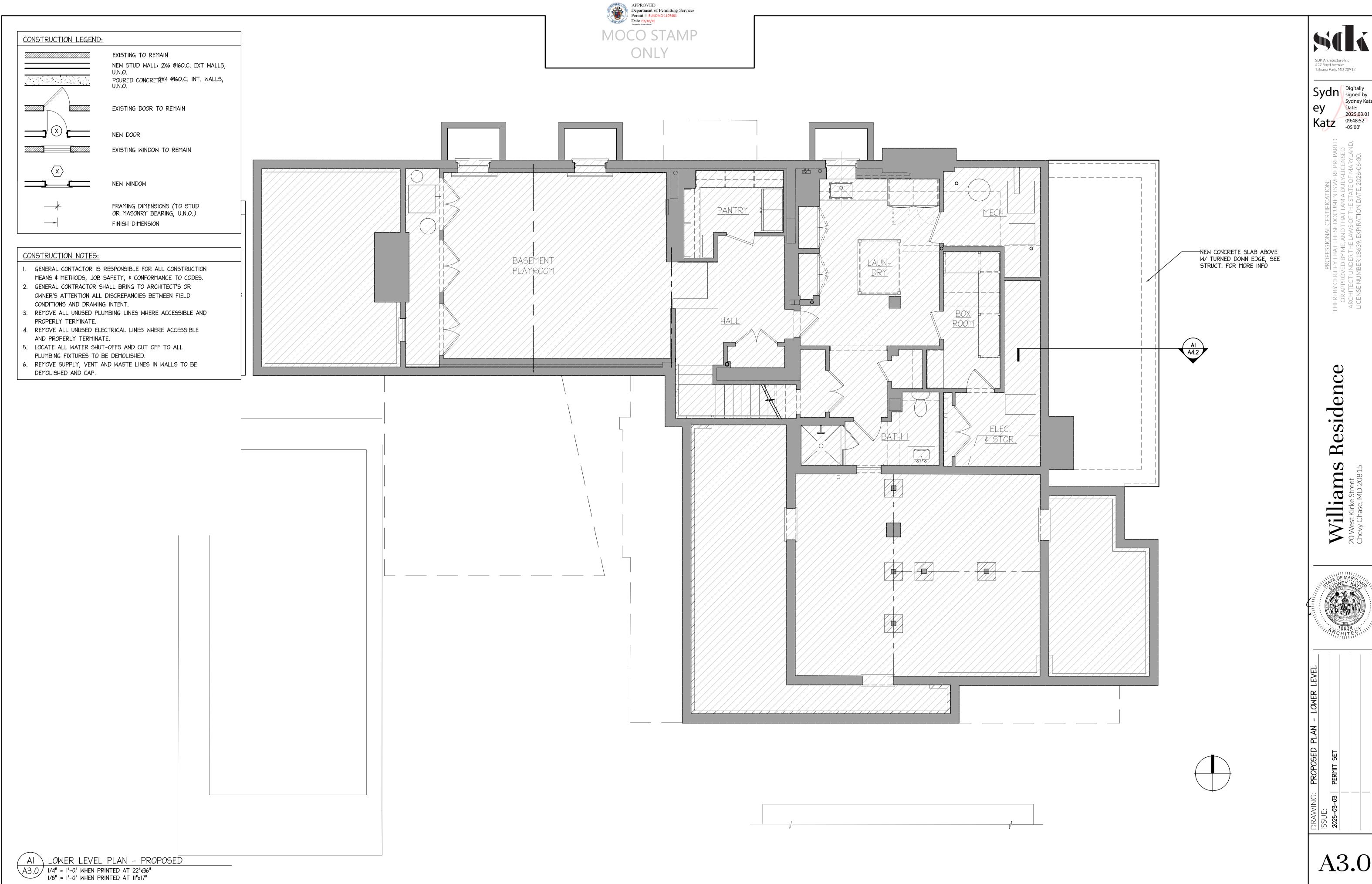


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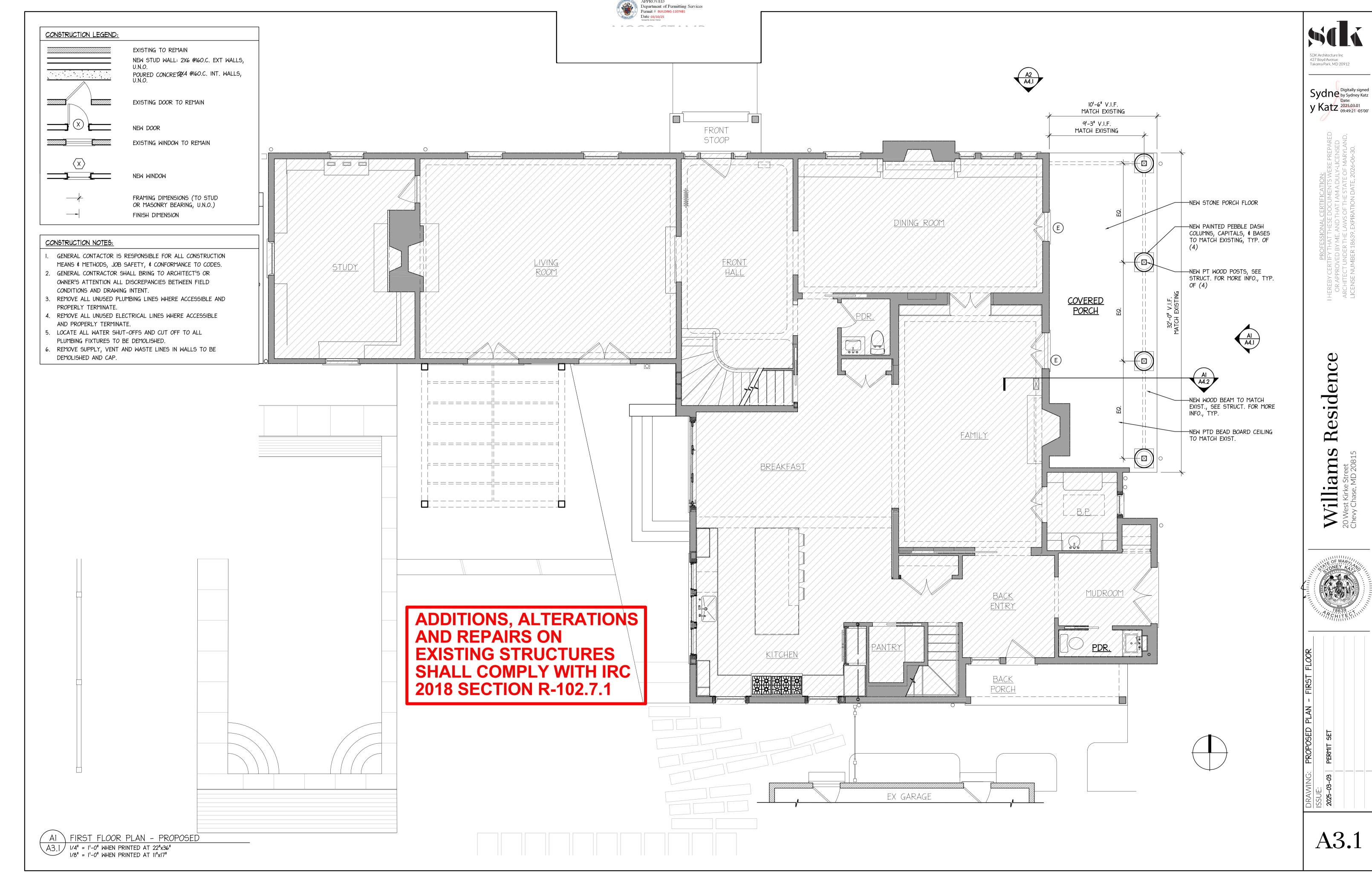


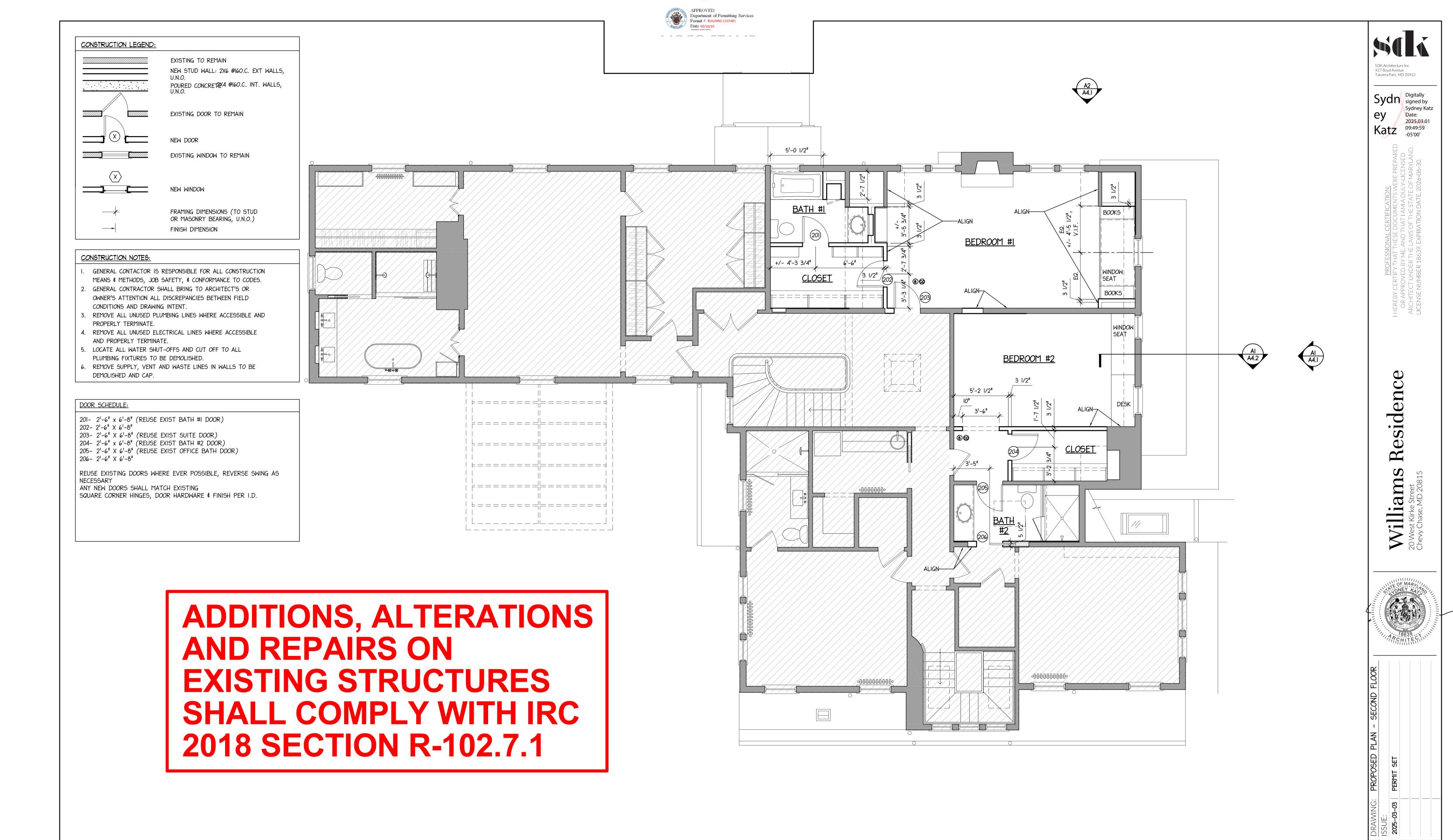
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Date: 2025.03.01 Katz 09:48:52 0-05'00'

A3.0





A1 SECOND FLOOR PLAN - PROPOSED

A3.2 1/4" = 1'-0" WHEN PRINTED AT 22"x36"
1/8" = 1'-0" WHEN PRINTED AT 11"x17"

A3.2



SDK Architecture Inc

SDK Architecture Inc 427 Boyd Avenue Takoma Park, MD 20912

Sydn Signed by Sydney Katz Date: 2025.03.01

Katz 09:50:55 -05'00'

EREBY CERTIFY THAT THESE DOCUMENTS WERE PREPARION APPROVED BY ME, AND THAT I AM A DULY-LICENSED SCHITECT UNDER THE LAWS OF THE STATE OF MARYLANI CENSE NUMBER 18639, EXPIRATION DATE, 2026-06-30.

Williams Residence
20 West Kirke Street
Chevy Chase, MD 20815



SECTION

JG: BUILDING SECTION
OB | PERMIT SET

1SSUE: | 2025-03-03 | PERI

A4.2





ICENSE NO. 43074 EXPIRATION DATE: 12/13/2026



CONTRACTOR SHALL PROVIDE TEMPORARY SHORING. BRACING. SHEETING AND MAKE SAFE ALL FLOORS. ROOFS. WALLS AND ADJACENT PROPERTY, AS PROJECT CONDITIONS REQUIRE. A PROFESSIONAL ENGINEER, LICENSED BY THE STATE OF MARYLAND AND HIRED BY THE CONTRACTOR, SHALL DESIGN ALL SHORING AND SHEETING AND SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS FOR THE OWNER'S REVIEW.

ALL STRUCTURAL WORK SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND SHALL CONFORM TO THE PROJECT SPECIFICATIONS, INCLUDING THE INTERNATIONAL RESIDENTIAL CODE (2018) AS MODIFIED

DIMENSIONS AND ELEVATIONS OF EXISTING CONSTRUCTION GIVEN IN STRUCTURAL DRAWINGS ARE BASED ON INFORMATION CONTAINED IN VARIOUS ORIGINAL DESIGN AND CONSTRUCTION DOCUMENTS PROVIDED BY THE OWNER, AND LIMITED FIELD OBSERVATIONS AND MEASUREMENTS. THE CONTRACTOR SHALL VERIFY ALL INFORMATION PERTAINING TO EXISTING CONDITIONS BY ACTUAL MEASUREMENT AND OBSERVATION AT THE SITE. ALL DISCREPANCIES BETWEEN ACTUAL CONDITIONS AND THOSE SHOWN IN THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT FOR EVALUATION BEFORE THE AFFECTED CONSTRUCTION IS PUT IN PLACE.

THE INFORMATION CONTAINED IN THIS SET OF DRAWINGS REPRESENTS THE DESIGN INTENT OF THE PROPOSED CONSTRUCTION. ELECTRONIC VERSIONS (PDF, DWG) OF THESE DRAWINGS SHOULD NOT BE USED TO DETERMINE DIMENSIONS OR GATHER ANY INFORMATION THAT IS NOT SPECIFICALLY LABELED OR OTHERWISE DENOTED IN PLAN, SECTION, OR DETAIL. DUPLICATION OF THESE DRAWINGS FOR USE IN THE PREPARATION OF SHOP DRAWINGS IS NOT ACCEPTABLE. THIS INCLUDES ANNOTATED HARD-COPIES AND DIRECT REUSE OF ELECTRONIC

BUILDING FOUNDATIONS SHALL BEAR ON UNDISTURBED SOIL HAVING MINIMUM BEARING CAPACITY OF 1500 PSF. ADEQUACY OF BEARING STRATUM SHALL BE VERIFIED IN FIELD PRIOR TO PLACING CONCRETE. ADJUST BOTTOM OF FOOTING ELEVATIONS AS REQUIRED.

FINISH ALL FOOTING EXCAVATIONS BY HAND. NO FOOTINGS SHALL BE PLACED IN WATER OR ON FROZEN GROUND.

PROTECT FOOTINGS FROM FROST AFTER THEY ARE PLACED. AT INTERSECTIONS BETWEEN NEW AND EXISTING WALLS, STEP NEW FOOTING TO MATCH EXISTING. DRILL AND GROUT 2-#5 BARS x 2'-6" LONG INTO EXISTING FOOTING IN HILTI HIT-HY200 ADHESIVE WITH 6" EMBEDMENT. 4. DO NOT PLACE FILL AGAINST FOUNDATION WALLS UNLESS ADEQUATELY BRACED BY COMPLETED FLOORS OR OTHER

MEANS DEEMED APPROPRIATE BY THE ARCHITECT. FILL AND BACKFILL MATERIAL— CLEAN RUN OF BANK MATERIAL, FREE OF DELETERIOUS ORGANIC MATERIALS.

6. ALL EXTERIOR FOOTINGS SHALL BE PLACED A MINIMUM OF 2'-6'' BELOW FINAL GRADE.

CAST-IN-PLACE CONCRETE

ALL CONCRETE SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 4500 PSI AT 28 DAYS. SLUMP SHALL BE 4" FOR SLABS ON GRADE AND 5" FOR ALL OTHER CONCRETE.

ALL FOUNDATION CONCRETE SHALL INCLUDE 5% AIR ENTRAINMENT  $(\pm 1.5\%)$ . ADJUST AIR ENTRAINMENT FOR EXPOSURE CLASS AS REQUIRED.

REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. REINFORCING STEEL SHALL BE DETAILED ACCORDING TO THE ACI MANUAL OF CONCRETE PRACTICE (ACI 315), LOCALLY APPROVED EDITION.

WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185, WITH A MINIMUM ULTIMATE TENSILE STRENGTH OF CONCRETE WORK SHALL BE DESIGNED, REINFORCED, PLACED AND CURED IN CONFORMANCE WITH THE LOCALLY

APPROVED EDITION OF ACI 301, "SPECIFICATIONS FOR STRUCTURAL CONCRETE", AND ALL RECOMMENDED PRACTICES CONTAINED THEREIN SHALL BE CONSIDERED MANDATORY FOR THIS PROJECT. PROVIDE MINIMUM TEMPERATURE REINFORCEMENT, AS REQUIRED BY ACI-318, IN ALL SLABS AND WALLS WHERE

REINFORCEMENT IS NOT INDICATED ON DRAWINGS. COORDINATE SIZE AND LOCATION OF ALL OPENINGS AND PIPE SLEEVES WITH ARCHITECTURAL AND MECHANICAL

DRAWINGS. MINIMUM CONCRETE BETWEEN SLEEVES SHALL BE 6". PROVIDE CLEARANCE FROM FACE OF CONCRETE TO REINFORCEMENT AS FOLLOWS:

BEAMS. COLUMNS:

FOOTINGS: EXTERIOR WALLS: 2" FOR #6 OR LARGER, 1 1/2" FOR #5 OR SMALLER

INTERIOR WALLS: ALL GROUT SHALL BE NON-SHRINK WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI. 10. UNLESS SPECIFICALLY WAIVED BY ENGINEER OF RECORD, CEMENTITIOUS MATERIAL REPLACEMENT FOR CONCRETE MIXES AT ALL CAST-IN-PLACE CONCRETE SHALL BE 10% MINIMUM AND 33% MAXIMUM USING ONE OF THE FOLLOWING: GROUND GRANULATED BLAST FURNACE SLAG (GGBFS) OR FLY ASH.

WHERE CONCRETE IS PLACED AGAINST AND DOWELED TO HARDENED CONCRETE AND/OR WHERE A ROUGHENED SURFACE IS INDICATED IN THE STRUCTURAL DRAWINGS, THE HARDENED CONCRETE SURFACE SHALL BE CLEAN AND FREE OF LAITANCE AND SHALL BE ROUGHENED TO A FULL AMPLITUDE OF APPROXIMATELY 1/4".

## WOOD STRUCTURAL PANEL SHEATHING

PROVIDE STRUCTURAL I PLYWOOD OR OSB SHEATHING WITH BOND CLASSIFICATIONS APPROPRIATE TO THE END USE: "EXTERIOR" (PERMANENT EXPOSURE), OR "EXPOSURE I" (CONSTRUCTION EXPOSURE ONLY) FLOOR SHEATHING: NOM. 3/4" THICK T & G PLYWOOD OR OSB (48/24 SPAN RATING), APA STURD-I-FLOOR, OR

ADVANTECH SUBFLOOR. ROOF SHEATHING (STANDARD): NOM. 5/8" THICK T & G PLYWOOD OR OSB (48/24 SPAN RATING). 4. ROOF SHEATHING (UNDER SLATE OR CLAY TILE): NOM. 3/4" THICK T & G PLYWOOD OR OSB (48/24 SPAN RATING).

WALL SHEATHING (STANDARD): NOM. 1/2" THICK PLYWOOD (32/16 SPAN RATING).

WALL SHEATHING (BEHIND SLATE, CLAY TILE, OR MASONRY VENEER): NOM. 3/4" THICK PLYWOOD (48/24 SPAN ALL FLOOR SHEATHING SHALL BE GLUED AND SCREWED TO FLOOR JOISTS USING AN APA APPROVED ADHESIVE (LOKTITE PL400 OR EQUAL).

8. USE PLY CLIPS OR OTHER EDGE SUPPORT AS REQUIRED FOR SHEATHING.

9. LEAVE 1/6" SPACE AT ALL PLYWOOD PANEL END JOINTS AND 1/8" SPACE AT ALL PLYWOOD PANEL EDGE JOINTS EXCEPT WHEN USING T & G PANELS 10. UNLESS NOTED OTHERWISE, WALL SHEATHING SHALL BE FASTENED TO FRAMING WITH 10d COMMON NAILS @ 4" O.C.

AT EACH SHEET PERIMETER AND 12" O.C. ELSEWHERE. PROVIDE 2x6 BLOCKING AT ALL FREE EDGES. 1. UNLESS NOTED OTHERWISE, FLOOR SHEATHING UP TO 3/4" THICK SHALL BE FASTENED TO FRAMING WITH 2-1/2" LONG SIMPSON WSNTL QUIK DRIVE SCREWS (0.175" DIA.), AND FLOOR SHEATHING GREATER THAN 3/4" SHALL BE FASTENED TO FRAMING WITH 3" LONG SIMPSON WSNTL QUIK DRIVE SCREWS. FLOOR

SHEATHING SHALL ALSO BE GLUED TO FRAMING USING AN APA-APPROVED ADHESIVE.

12. UNLESS NOTED OTHERWISE, ROOF SHEATHING SHALL BE FASTENED TO FRAMING WITH 10d COMMON NAILS. 13. UNLESS NOTED OTHERWISE, FLOOR AND ROOF DIAPHRAGMS SHALL BE UNBLOCKED.

A. UNBLOCKED DIAPHRAGMS: UNLESS NOTED OTHERWISE, FASTENERS OF SHEATHING TO FRAMING SHALL BE SPACED @ 6" O.C. AT SUPPORTED SHEATHING PANEL EDGES AND AT ALL DIAPHRAGM BOUNDARIES (PERIMETER OF FLOOR/ROOF; PERIMETER OF ALL OPENINGS; AND ALL RIDGES, VALLEYS, HIPS, AND OTHER CHANGES IN SLOPE)

B. BLOCKED DIAPHRAGMS: UNLESS NOTED OTHERWISE, FASTENERS OF SHEATHING TO FRAMING SHALL BE SPACED @ 6" O.C. AT ALL SHEATHING PANEL EDGES AND @ 12" O.C. ELSEWHERE. PROVIDE 2x BLOCKING AT ALL UNSUPPORTED PANEL EDGES TO RECEIVE FASTENERS.

1. FRAMING LUMBER SHALL HAVE EACH PIECE GRADE STAMPED, SHALL BE SURFACED DRY (EXCEPT STUDS, WHICH SHALL BE KILN-DRIED) AND SHALL CONFORM TO THE FOLLOWING SPECIES AND GRADE: RAFTERS AND JOISTS: HEM-FIR #2 OR SPRUCE-PINE-FIR #2

BEAMS, GIRDERS AND HEADERS: HEM-FIR #1 OR SPRUCE-PINE-FIR #1 STUDS AND PLATES: HEM-FIR STUD GRADE OR SPRUCE-PINE-FIR STUD GRADE TIMBER LUMBER SHALL CONFORM TO THE FOLLOWING SPECIES AND GRADE:

POST AND TIMBER: HEM-FIR #1 OR SPRUCE-PINE-FIR #1

BEAMS AND STRINGERS: HEM-FIR #1 OR SPRUCE-PINE-FIR #1 PRESERVATIVE—TREATED WOOD: PROVIDE TREATED SOUTHERN PINE #2 LUMBER COMPLYING WITH ACQ-D (CARBONATE), COPPER AZOLE (CA-B), OR SODIUM BORATE (SBX (DOT) WITH NaSIO2) AT ALL LUMBER IN CONTACT WITH CONCRETE OR MASONRY, OR AS OTHERWISE INDICATED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. ACZA TREATMENT IS NOT PERMITTED. TREATED LUMBER AND/OR PLYWOOD SHALL BEAR THE LABEL OF AN ACCREDITED AGENCY SHOWING 0.40 PCF RETENTION. WHERE LUMBER AND/OR PLYWOOD IS CUT OR DRILLED AFTER TREATMENT, THE TREATED SURFACE SHALL BE FIELD-TREATED WITH COPPER NAPTHENATE (THE CONCENTRATION OF WHICH SHALL CONTAIN A MINIMUM OF 2% COPPER METAL) BY REPEATED BRUSHING. DIPPING, OR SOAKING UNTIL THE WOOD ABSORBS NO MORE PRESERVATIVE.

4. ALL WOOD FRAMING INCLUDING DETAILS FOR BRIDGING, BLOCKING, FIRE STOPPING, ETC., SHALL CONFORM TO THE LOCALLY APPROVED EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND ITS SUPPLEMENTS AND SHALL BE INSTALLED IN ACCORDANCE WITH THE INTERNATIONAL RESIDENTIAL CODE (SEE DESIGN LOADS AND FACTORS TABLE FOR IRC EDITION).

5. FASTENING SHALL BE IN ACCORDANCE WITH THE MOST RESTRICTIVE OF: THE INTERNATIONAL RESIDENTIAL CODE. OR THE MANUFACTURER'S RECOMMENDED FASTENING SCHEDULES. (SEE DESIGN LOADS AND FACTORS TABLE

6. ALL FLUSH FRAMED CONNECTIONS SHALL BE MADE WITH APPROVED GALVANIZED STEEL JOIST OR BEAM HANGERS. MINIMUM 18 GAUGE. INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. 7. WHERE FRAMING LUMBER IS FLUSH FRAMED TO MICROLLAM, STEEL OR FLITCH-PLATE GIRDER, SET THESE

GIRDERS ¼" CLEAR (MIN.) BELOW TOP OF FRAMING LUMBER, TO ALLOW FOR SHRINKAGE.

8. STUD BEARING WALLS ARE TO BE 2x6, @ 16" O.C., UNLESS NOTED OTHERWISE ON PLAN. . LAP ALL PLATES AT CORNERS AND AT INTERSECTION OF PARTITIONS.

10. STAGGER ALL TOP AND BOTTOM PLATE SPLICES A MINIMUM OF 32 INCHES.

11. USE DOUBLE STUDS @ ENDS OF WALL AND ENDS OF WALL OPENINGS.

12. AT THE ENDS OF ALL BEAMS, HEADERS AND GIRDERS PROVIDE A BUILT UP OR SOLID POST WHOSE WIDTH IS AT LEAST EQUAL TO THE WIDTH OF THE MEMBER IT IS SUPPORTING AND WHOSE DEPTH IS 4" (NOM.) AT INTERIOR WALLS AND 6" (NOM.) AT EXTERIOR WALLS.

13. USE DOUBLE TRIMMERS AND HEADERS AT ALL FLOOR OPENINGS WHERE BEAMS ARE NOT DESIGNATED. 14. BRIDGING FOR SPANS UP TO 14 FT., PROVIDE 1 ROW. BRIDGING FOR SPANS OVER 14 FT., PROVIDE 2 ROWS.

15. BUILT-UP BEAMS LESS THAN 8" DEEP SHALL BE SPIKED TOGETHER WITH (2) 16D NAILS @ 16" O.C. BUILT-UP BEAMS GREATER THAN 8" DEEP SHALL BE SPIKED TOGETHER WITH (3) 16D NAILS @ 16" O.C.

16. WHERE THERE IS NO PLYWOOD WALL SHEATHING, PROVIDE DIAGONALS AT ALL EXTERIOR CORNERS OF STUD WALLS AT EACH FLOOR. (1"x4" BRACES LET INTO STUDS AND NAILED AT EACH STUD CROSSING WITH (2) 10D

17. WHERÉ CANTILEVERED BEAMS ARE INDICATED, THE FAR CONNECTOR SHALL BE CAPABLE OF RESISTING AN UPLIFT OF 1000 LBS. MIN., U.N.O.

18. NO NEW OR EXISTING JOISTS SHALL BE CUT OR NOTCHED WITHOUT APPROVAL.

19. ALL LIGHT-GAGE HANGERS SUPPORTING PRESERVATIVE TREATED WOOD SHALL MEET OR EXCEED G185 (1.85 oz OF ZINC PER SQUARE FOOT). ALTERNATIVELY, STAINLESS STEEL CONNECTORS MAY BE USED. FASTENERS SHALL

MATCH THE SELECTED HANGER FINISH AND MATERIAL 20. WHERE JOIST ORIENTATION IS PARALLEL TO EXTERIOR STUD OR FOUNDATION WALLS, PROVIDE FULL-SECTION BLOCKING FOR 3 BAYS @ 4'-0" O.C. MAX.

A. WHERE SHEATHING IS NOT CONTINUOUSLY FASTENED TO TOP OF JOISTS, PROVIDE 18 GA.x 11/2"x12" (MIN.) FLAT TENSION STRAPS BETWEEN ALIGNED BLOCKING MEMBERS.

B. WHERE SHEATHING IS NOT CONTINUOUSLY FASTENED TO BOTTOM OF JOISTS, PROVIDE 18 GA.x 11/2"x12" (MIN.) FLAT TENSION STRAPS BETWEEN ALIGNED BLOCKING MEMBERS.

22. ALL SILL PLATES SHALL BE P.T. AND ANCHORED TO FOUNDATION WALLS W/ ½" DIA. HEADED ANCHOR BOLTS (ASTM F1554) @ 4'-0" O.C. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION WITH (1) BOLT LOCATED NOT MORE THAN 12" OR LESS THAN 7x BOLT DIA. FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL HAVE A MINIMUM 7" EMBEDMENT INTO CONCRETE OR GROUTED CMU CELLS. THE BOLTS SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE PLATE WIDTH AND HAVE A TIGHTENED NUT AND

23. WOOD BEAMS, JOIST. STUDS AND OTHERS COMBUSTIBLE MATERIAL SHALL HAVE A CLEARANCE OF NOT LESS THAN 2 INCHES (51 mm) FROM THE FRONT AND SIDES OF MASONRY FIREPLACES AND NOT LESS THAN 4 INCHES (102 mm) FROM THE BACK FACES OF MASONRY FIREPLACES. THE AIRSPACES SHALL NOT BE FILLED, EXCEPT TO PROVIDE FIREBLOCKING WITH SECTION R1001.12.

## **WOOD HEADER SCHEDULE**

1. UNLESS NOTED OTHERWISE IN PLAN, PROVIDE HEADERS PER THE FOLLOWING:

ROUGH OPENING WIDTH:	HEAD	ER:	JACK STUDS	KING	STUDS
	2x4 WALL	2x6 WALL	ALL	INTERIOR	<b>EXTERIOR</b>
LESS THAN 3'-0"	(2) 2x6	(3) 2x8	1	1	1
3'-1 TO 4'-0"	(2) 2x8	(3) 2x8	1	1	2
4'-1" TO 6'-0"	(2) 2x10	(3) 2x10	2	2	2
6'-1" TO 8'-0"	(2) 2x12	(3) 2×12	2	2	3
OVFR 8'-0"		`´	FF PLANS		

NOTES: AT FLUSH FRAMED BEAMS PROVIDE THE REQUIRED NUMBER OF KING STUDS NOTED, 2 MINIMUM.

## **ENGINEERED WOOD PRODUCTS**

1. WOOD I-JOISTS: PROVIDE ENGINEERED WOOD I-JOISTS, SIZES AND SERIES AS SHOWN, AS MANUFACTURED BY WEYERHAUSER OR APPROVED EQUAL. INSTALL IN STRICT COMPLIANCE WITH THE MANUFACTURER'S STANDARD RECOMMENDATIONS AND DETAILS, INCLUDING CONSTRUCTION BRACING, MINIMUM BEARING LENGTHS, WEB STIFFENERS, SQUASH BLOCKS, BLOCKING, KNOCKOUTS AND HOLES, ETC. THE JOIST SPACING IDENTIFIED ON PLAN MAY BE EXCEEDED AT ISOLATED LOCATIONS TO ACCOMMODATE THE WORK OF OTHER TRADES PROVIDED THE FOLLOWING

A. THE SUM OF TWO ADJACENT JOISTS SPACINGS SHALL NOT EXCEED TWO TIMES THE AVERAGE SPACING SHOWN ON PLAN. B. NO SINGLE JOIST SPACING SHALL EXCEED 21"

2. RIM BOARDS: PROVIDE CONTINUOUS 11/4" THICK RIM BOARDS, TIMBERSTRAND LSL AS MANUFACTURED BY WEYERHAUSER, OR APPROVED EQUAL. INSTALL IN COMPLIANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AT THE PERIMETER OF ALL FLOOR PLATFORMS.

3. MICROLLAM BEAMS: PROVIDE ENGINEERED BEAMS, SIZES AS SHOWN, MICROLLAM LVL (Fb=2600 PSI, E=2,000,000 PSI) OR PARALLAM PSL (Fb=2900 PSI, E=2,000,000 PSI) AS MANUFACTURED BY WEYERHAUSER OR APPROVED EQUAL. INSTALL IN STRICT COMPLIANCE WITH THE MANUFACTURER'S STANDARD RECOMMENDATIONS AND DETAILS.

4. GLUED LAMINATED TIMBER (SOFTWOOD): PROVIDE ENGINEERED BEAMS, SIZES AS SHOWN, IN ACCORDANCE WITH AITC 117-04 DESIGN STANDARD SPECIFICATIONS FOR STRUCTURAL GLUED LAMINATED TIMBER OF SOFTWOOD SPECIES. UNLESS NOTED OTHERWISE, ALL LAMINATIONS SHALL BE SOUTHERN PINE.

A. ANTHONY POWER COLUMNS: COMBINATION 50 SOUTHERN PINE N1D14

B. ANTHONY POWER PRESERVED COLUMNS: COMBINATION 50 SOUTHERN PINE N1D14

C. ANTHONY POWER BEAMS: 3000 Fb - 2.1E - 300 Fv D. ANTHONY POWER PRESERVED BEAMS: 24F-V5M1/SP (2400 Fb - 1.8E - 300 Fv) 5. WHERE JOIST ORIENTATION IS PARALLEL TO EXTERIOR STUD OR FOUNDATION WALLS, PROVIDE FULL-SECTION

BLOCKING FOR 3 BAYS @ 4'-0" O.C. MAX. B. WHERE SHEATHING IS NOT CONTINUOUSLY FASTENED TO TOP OF JOISTS, PROVIDE 18 GA.X 11/2"x12" (MIN.) FLAT TENSION STRAPS BETWEEN ALIGNED BLOCKING MEMBERS. C. WHERE SHEATHING IS NOT CONTINUOUSLY FASTENED TO BOTTOM OF JOISTS, PROVIDE 18 GA.X 1½"X12" (MIN.)

FLAT TENSION STRAPS BETWEEN ALIGNED BLOCKING MEMBERS. S. USE DOUBLE TRIMMERS AND HEADERS AT ALL FLOOR OPENINGS WHERE BEAMS ARE NOT DESIGNATED.

7. BRIDGING FOR SPANS UP TO 14 FT., PROVIDE 1 ROW. BRIDGING FOR SPANS OVER 14 FT., PROVIDE 2 ROWS.

#### **INSPECTION AND TESTING**

- 1. THE FOLLOWING MINIMUM INSPECTIONS SHALL BE PERFORMED BY A TESTING AGENCY ENGAGED BY THE OWNER. ADDITIONAL INSPECTIONS MAY BE REQUIRED BY THE LOCAL JURISDICTION'S SPECIAL INSPECTIONS PROGRAM
- (SEE ITEM 2).: A. WELDING
- B. SUBGRADE FOR FOUNDATIONS

DOCUMENTS AND SHOP DRAWINGS

- C. HIGH STRENGTH BOLTING D. QUALITY CONTROL OF CONCRETE MATERIALS, BATCHING, STRENGTH, SLUMP, AIR CONTENT, UNIT WEIGHT, TEMPERATURE, FORMS, SIZE AND PLACEMENT OF REINFORCEMENT.
- E. STABILITY OF BUILDING CONSTRUCTION. 2. WHERE REQUIRED BY THE LOCAL JURISDICTION, A SEPARATE SCHEDULE OF INSPECTIONS WILL BE COMPLETED IN ACCORDANCE WITH THE JURISDICTION'S REQUIREMENTS. THE TESTING AGENCY SHALL FILE THIS SCHEDULE
- AND ALL OTHER NECESSARY FORMS WITH THE BUILDING DEPARTMENT 3. CAST-IN-PLACE CONCRETE (IF NO SPECIFICATION DOCUMENT PROVIDED): A. INSPECT THE FORMWORK AND REINFORCING STEEL PLACEMENT FOR COMPLIANCE WITH THE CONTRACT
  - B. MONITOR STRUCTURAL CONCRETE PLACEMENT FOR CONFORMANCE BASED ON ACI STANDARDS C. AT THE TIME OF CONCRETE PLACEMENT, CAST CYLINDERS AND TAKE COMPOSITE CONCRETE SAMPLES FOR THE PURPOSES OF TESTING AIR ENTRAINMENT, SLUMP, DENSITY, AND COMPRESSIVE STRENGTH AS

1. SAMPLE FRESH CONCRETE IN ACCORDANCE WITH ASTM C172. MOLD TEST CYLINDERS IN ACCORDANCE WITH ASTM C31.

2. CAST AND LAB CURE THE FOLLOWING NUMBER OF TEST CYLINDERS FOR EACH DAY'S POUR OR EACH 50 CUBIC YARDS, WHICHEVER RESULTS IN MORE TEST CYLINDERS:

2 FOR 7-DAY TEST – 3 FOR 28–DAY TEST,

- 1 HELD FOR CHECKING LOW BREAK RESULTS.

3. FIELD-CURED CYLINDERS SHALL BE CAST FOR HOT AND COLD WEATHER CONCRETE PLACEMENTS (2) FOR 7-DAY AND 2 FOR 28-DAY). COLD WEATHER CONCRETE PLACEMENTS OCCUR WHEN THE AVERAGE EXPECTED AIR TEMPERATURES FOR 3 CONSECUTIVE DAYS FOLLOWING THE PLACEMENT ARE LESS THAN 40 DEGREES, RESPECTIVELY. HOT WEATHER CONCRETE PLACEMENTS OCCUR WHEN THE

AIR TEMPERATURE AT THE TIME OF PLACEMENT EXCEEDS 90 DEGREES 3. FOR LIGHTWEIGHT CONCRETE, TESTS SHALL BE MADE TO VERIFY THAT THE CONCRETE DENSITY CONFORMS TO THE RANGE OF 110-115 PCF (NOT REQUIRED FOR NORMAL WEIGHT CONCRETE). 4. MAKE ADDITIONAL TESTS OF IN-PLACE CONCRETE AT THE CONTRACTOR'S EXPENSE WHEN CYLINDER

TEST RESULTS INDICATE SPECIFIED CONCRETE STRENGTHS HAVE NOT BEEN ATTAINED (DIRECTED BY THE A/E TEAM), OR WHEN REQUESTED BY THE CONTRACTOR FOR EARLY FORMWORK REMOVAL. 4. STRUCTURAL STEEL (IF NO SPECIFICATION DOCUMENT PROVIDED):

B. MONITOR THE INSTALLATION OF BOLTS REQUIRING PRE-TENSIONING FOR CONFORMANCE WITH SPECIFIC PRE-CALIBRATED TIGHTENING PROCEDURES.

C. PERFORM WELDING INSPECTION AND TESTING PROCEDURES IN ACCORDANCE WITH THE AWS CODE. 1. TEST EACH FULL PENETRATION BUTT OR GROOVE WELD AND ALL PARTIAL PENETRATION WELDS, AS WELL AS ANY SUSPECT POOR QUALITY FILLET WELD PER ONE OF THE FOLLOWING PROCEDURES:

a. LIQUID PENETRANT INSPECTION: ASTM E 165. RESERVE THIS TEST FOR FILLET WELDS ONLY. b. MAGNETIC PARTICLE INSPECTION: ASTM E 709; PERFORMED ON ROOT PASS AND ON FINISHED WELDS. CRACKS OR ZONES OF INCOMPLETE FUSION OR PENETRATION ARE NOT ACCEPTED.

c. ULTRASONIC INSPECTION: ASTM E 164. d. RADIOGRAPHIC INSPECTION: ASTM E 94

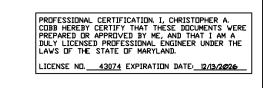
A. VISUALLY INSPECT ALL FILLET WELDS, AND BOLTED CONNECTIONS.

PLANS REVIEWED AS PERFORMANCE **COMPLIANCE WITH IRC 2018 SECTION** R-301.1.3. AND APPLICABLE ENGINEERING DESIGN STANDARDS.

S001







DESIGN LOADS AND FACTORS  DESIGN CODE: 2018 IRC AS MODIFIED BY THE LOCAL JURISDICTION															
LIVE LOAD DATA		ROOF LOAD DATA		DEAD LOAD DATA		WIND LOAD DATA		EARTHQUAKE DESIGN DATA		SOIL DESIGN DATA		DEFLECTIONS LIMITS FOR WOOD FRAMING			
FLOOR OR ROOF AREA	LOAD (PSF)	LOAD TYPE	VALUE (PSF)	AREA	VALUE (PSF)	PARAMETER	VALUE	PARAMETER	VALUE	PARAMETER	VALUE		LL	TL	Δπ.(in)
TYP. FLOOR (U.N.O.)	40	GROUND SNOW LOAD (Pg)	30	FLOOR	15	ULTIMATE DESIGN WIND SPEED	115 MPH	SHORT-PERIOD MAP VALUE (S <sub>S</sub> )	15.0% g	AT-REST PRESSURE CONDITION	65 PSF/FT	RAFTERS	L/360	L/240	0.75
EXTERIOR BALCONIES	60	NON-DRIFT SNOW	30	PARTITION	10	WIND EXPOSURE	В	SEISMIC SITE CLASS	D	ACTIVE PRESSURE CONDITION	45 PSF/FT	ROOF BEAMS	L/240	L/180	0 0.75
DECKS	40	DRIFTING SNOW	PER CODE	ROOF	15	IMPORTANCE FACTOR	1.0	SHORT-PERIOD DESIGN SPECTRAL	16.0% g	PASSIVE PRESSURE CONDITION	180 PSF/FT	JOIST	L/480	L/360	0.625
STAIRS	40							RESPONSE ACCELERATION (S <sub>DS</sub> )		SURCHARGE LOADS	100 PSF	FLOOR BEAMS	L/360	L/240	0.75
SLEEPING ROOMS	30							RESIDENTIAL SEISMIC DESIGN CATEGORY	А	S.O.G. COEFFICIENT OF SLIDING FRICTION	0.3	JOISTS/BEAMS-TILE OR STONE FINISH	L/600	L/480	0.5
ATTICS WITH STORAGE	20					SHEAR WALL TYPE	1	PER R301.2.2, THE SEISMIC PROVISIONS		FACTORS OF SAFETY (OTM & SLIDING)	1.5	MASONRY LINTELS (OR XFER BEAMS		1	
ATTICS WITHOUT STORAGE	10					CS-WSP (U.N.O.)		CODE ARE NOT APPLICABLE TO DETACHED ONE—FAMILY DWELLINGS ASSIGNED TO SEISMIC DESIGN CATEGORY A, B, OR C.		TOTAL/DIFFERENTIAL SETTLEMENT	1/.5 INCH	OF EXIST MASONRY)	L/600	L/600	0.3

<u>LEGEND</u>								
[]	EXIST. CONCRETE FOOTING	<u> </u>	WOOD JOIST					
	CONCRETE FOOTING	0	WOOD RAFTER					
	EXIST. BRICK MASONRY	2K/2J 2J/2K	WOOD BEAM, #J INDICATES NO. OF JACK STUDS, #K INDICATES NO. OF KING STUDS					
	BRICK MASONRY		WOOD HEADER					
	EXIST. CONCRETE MASONRY (CMU)		STEEL BEAM					
	CONCRETE MASONRY (CMU)	_··-	INDICATES EXIST. WOOD POST THRU OR DOWN					
<u>а.</u>	EXIST. CONCRETE WALL	—··-⊡—··-	INDICATES EXIST. WOOD POST ABOVE					
	CONCRETE WALL	•	INDICATES EXIST. STEEL POST THRU OR DOWN					
	EXIST. WOOD BEARING WALL		INDICATES EXIST. STEEL POST UP					
	WOOD BEARING WALL (2x6 @ 16" U.N.O.)	— n—	INDICATES WOOD POST THRU OR DOWN (APC POSTS SUPPORTING GIRDERS TO BE CONTINUOUS THROUGH					
	WALL BELOW TO BE REMOVED	_	FLOOR CONSTRUCTION DOWN TO THE FOUNDATION LEVEL)					
	BEARING WALL ABOVE	<del></del>	INDICATES WOOD POST ABOVE (REFER TO NOTES FOR WOOD POST THRU OR DOWN)					
<u> </u>	EXIST. WOOD JOIST		INDICATES STEEL POST UP					
0	EXIST. WOOD RAFTER		INDICATES STEEL POST THRU OR DOWN					
	EXIST. WOOD BEAM		DENOTES CONNECTION REQUIREMENTS (SEE SCHED.)					
	EXIST. WOOD FRAMING TO BE REMOVED	(X)	INDICATES TOP OF FOOTING ELEVATION					
	EXIST. STEEL BEAM	(##'-## <b>"</b> )	INDICATES FOR TOUTING ELEVATION					

ADD'L	ADDITIONAL	L.L.	LIVE LOAD
ADJ.	ADJACENT	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
A/E		LSL	LAMINATED STRAND LUMBER
	ALTERNATIVE	LVL	LAMINATED VENEER LUMBER
APC	ANTHONY POWER COLUMN		
APPROX.	APPROXIMATE	L-W	LONG WAY
ARCH.	ARCHITECTURAL/ARCHITECT	L.P.	LOW POINT
B.O.	BOTTOM OF	L.W.	LIGHT WEIGHT
BLDG.	BUILDING	MAX.	MAXIMUM
		MECH.	MECHANICAL
BM		MEP	MECHANICAL, ELECTRICAL, PLUMBIN
BOT.	BOTTOM		F.P.
BRG		MFR.	MANUFACTURER
BSMT	BASEMENT	MIN.	MINIMUM
CANT.	CANTILEVERED		
(C.E.)	CONCRETE ENCASED MEMBER	MISC.	MISCELLANEOUS
CFS		M.O.	MASONRY OPENING
C.I.		N.F.	NEAR FACE
C.I.P.	CAST IN PLACE	N.I.C.	NOT IN CONTRACT
		NO.	NUMBER
C.J.		NOM.	NOMINAL
CLG		N.S.	NEAR SIDE
CLR		N.T.S.	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	0.C.	ON CENTER
COL.	COLUMN		
CONC.	CONCRETE	0.D.	OUTSIDE DIAMETER
COORD.		0.F.	OUTSIDE FACE
CONTR.		OPNG.	OPENING
		OPP.	OPPOSITE
COTR.		P.A.F.	POWER ACTUATED FASTENER
CTR.	CENTER	PC.	PIECE
D.B.A.	DEFORMED BAR ANCHOR	P/C	PRECAST CONCRETE
DBL	DOUBLE	PERP.	PERPENDICULAR
DEMO	DEMOLITION		
DTL	DETAIL	PL.	PLATE
DIA.	DIAMETER	PLF	POUND PER LINEAR FOOT
DIAG.	DIAGONAL	PSI	POUND PER SQUARE INCH
DIM.	DIMENSION	PSL	PARALLEL STRAND LUMBER
		P-T	POST TENSIONED
D.L.	DEAD LOAD	P.T.	PRESERVATIVE TREATED
DN	DOWN	REINF.	REINFORCED
DO	DITTO	REQ'D	REQUIRED
DWG(S)	DRAWING(S)	REV.	REVISION
DWL	DOWEL		
(E)	EXISTING MEMBER OR DIMENSION	R.O.	ROUGH OPENING
EXIST.	EXISTING	SCHED.	SCHEDULE
		SECT.	SECTION
EA.	EACH	SIM.	SIMILAR
E/	EDGE OF	S.I.F.	STEP IN FOOTING
E.A.	EACH FACE	S.O.G	SLAB ON GRADE
E.J.	EXPANSION JOINT	SPEC.	SPECIFICATION
E.L.	ELEVATION	SQR.	SQUARE
EMBED.	EMBEDMENT	S.S.	STAINLESS STEEL
ENGR	ENGINEER		
E.O.R.	ENGINEER OF RECORD	STD.	STANDARD
		STIFF.	STIFFENER
EQ.	EQUAL SIDE	STIR.	STIRRUP
E.S.	EACH SIDE	STL.	STEEL
EXT.	EXTERIOR	SQR.	SQUARE
E.W.	EACH WAY	S-W	SHORT WAY
FNDN	FOUNDATION	SYM.	SYMMETRICAL
FIN.	FINISH	T.C.	TERRA COTTA
FLR.	FLOOR	T.O.	TOP OF
FRMG	FRAMING	T&B	TOP AND BOTTOM
F.S.	FAR SIDE		
		TEMP.	TEMPORARY
FTG	FOOTING	T&G	TOUNGE AND GROOVE
F.P.	FIRE PROTECTION	THK.	THICK(NESS)
F.W.	FLAT WISE	T.L.S.	TENSION LAP SPLICE
GA.	GAUGE	TR.	TRANSFER
GALV.	GALVANIZE	TYP.	TYPICAL
G.B.	GRADE BEAM	U.N.O.	UNLESS NOTED OTHERWISE
G-LAM	GLUE LAMINATED LUMBER		
HORIZ.	HORIZONTAL	U-P	UNDERPINNING
H.P.	HIGH POINT	VERT.	VERTICAL
		V.I.F.	VERIFY IN FIELD
HT.	HEIGHT	W/	WITH
HVAC	HEATING, VENTILATION & AIR	w.A.	WORK POINT
	CONDITIONING	₩-P	WATER PROOF
I.D.	INSIDE DIAMETER	W-F WWF	WELDED WIRE FABRIC
I.F.	INSIDE FACE		
l.J.	ISOLATION JOINT	#	NUMBER
	INFORMATION	<u> </u>	CENTER LINE
INF∩	11311 3 413 134 (43.1.4		
INFO. INT.	INTERIOR	Ø	DIAMETER

S002

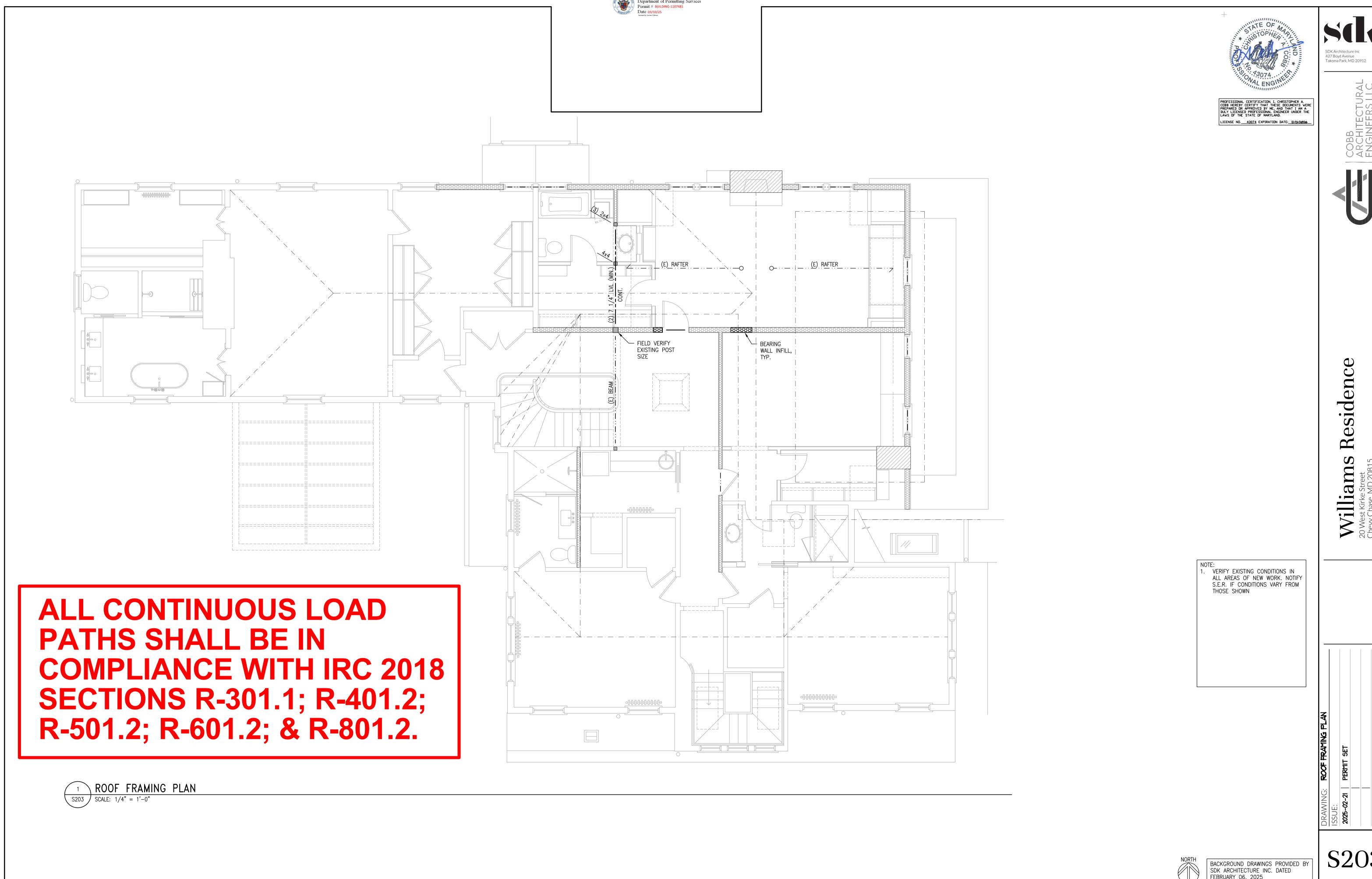
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**COMPLIANCE WITH IRC** 

VERIFY EXISTING CONDITIONS IN ALL AREAS OF NEW WORK. NOTIFY S.E.R IF CONDITIONS VARY
FROM THOSE SHOWN
APPB = ANTHONY POWER
PRESERVED BEAM

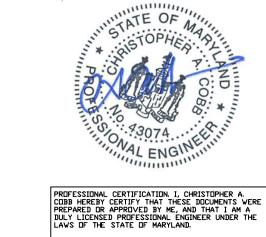
BACKGROUND DRAWINGS PROVIDED BY SDK ARCHITECTURE INC. DATED

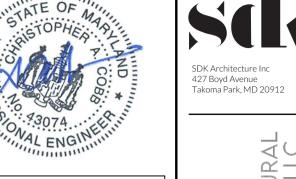




FEBRUARY 06, 2025







LICENSE NO. 43074 EXPIRATION DATE: 12/13/2026

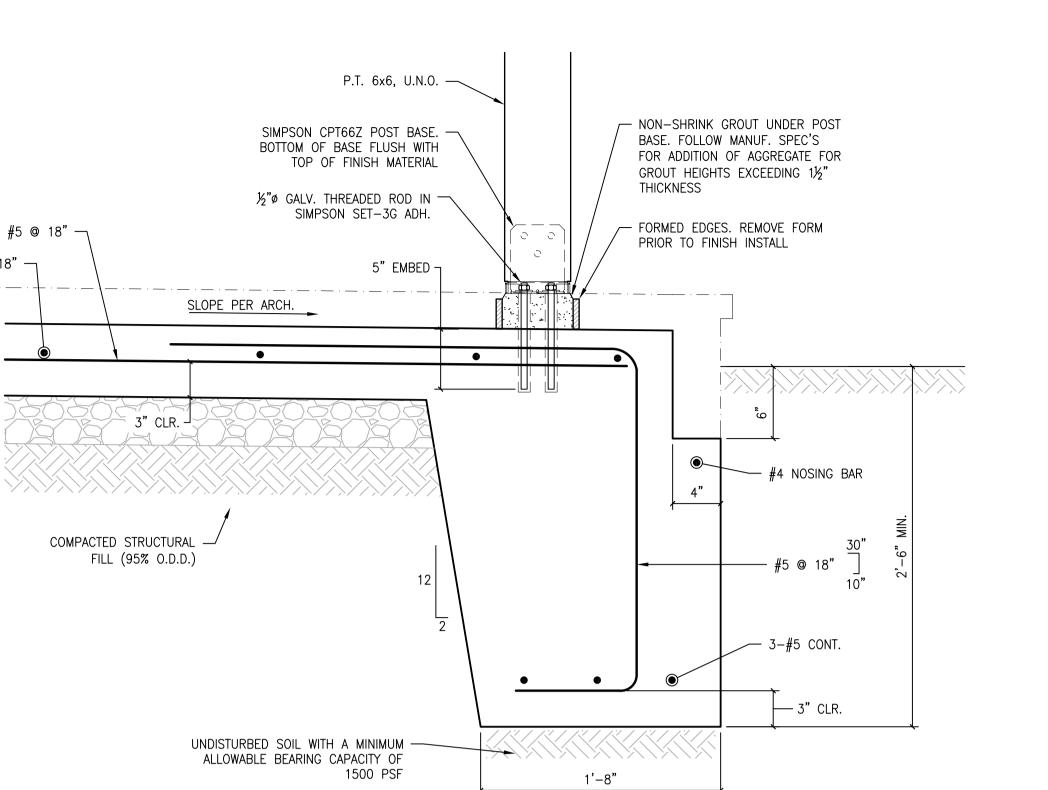
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\_\_\_\_ 2x6 BRG. WALL 2x6 BRG. WALL - 2x10 LEDGER — 2x8 LEDGER (3) SIMPSON .22"x6" SDWS (4) SIMPSON .22"x6" SDWS SCREWS INTO (1) STUD @ SCREWS INTO (1) STUD @ 16" O.C. MAX. 16" O.C. MAX.

- 1. SCREWS SHALL BE CENTERED ON WIDTH OF STUD
- 2. STUD WIDTH SHALL NOT BE LESS THAN 1½" 3. WHERE SCREWS ARE LOCATED WITHIN THE END 6" OF THE LEDGER, PREDRILL HOLE WITH  $\frac{5}{2}$ " BIT
- 4. WHERE 2x10 SOUTHERN PINE LEDGERS ARE SPECIFIED, ONLY INSTALL TOP (3) SCREWS

CASE I CASE II CASE I CASE I

41 26

59 38

34

13

17

22

14

23

27

40

DEFORMED BAR TENSION DEVELOPMENT LENGTH (Ld)

15

24

29

42

63

CASE I CASE II

26

45

23

30

38

46

67

60

88

101

112

127

141

41

59

67

85

94

CASE I CASE II

3000 PSI

CASE II

42

50

72

65

108

121

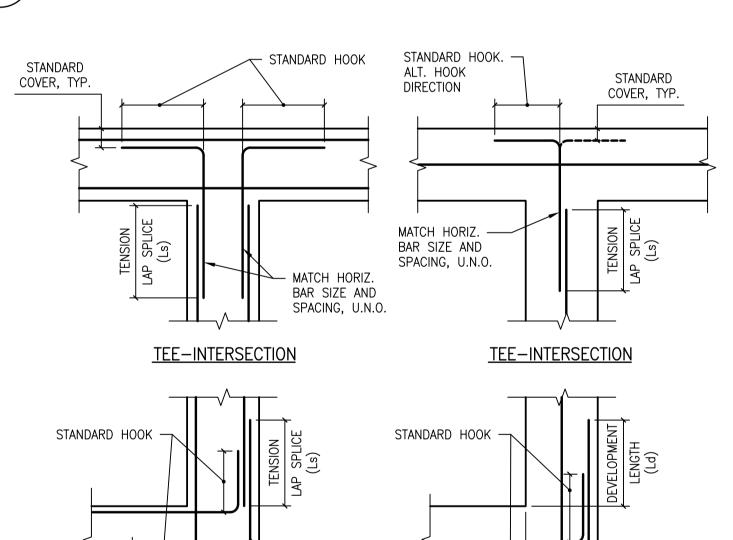
151

102

CASE I

48

STANDARD DETAIL: LEDGER ATTACHMENT TO WOOD WALL S300 / SCALE: N.T.S.



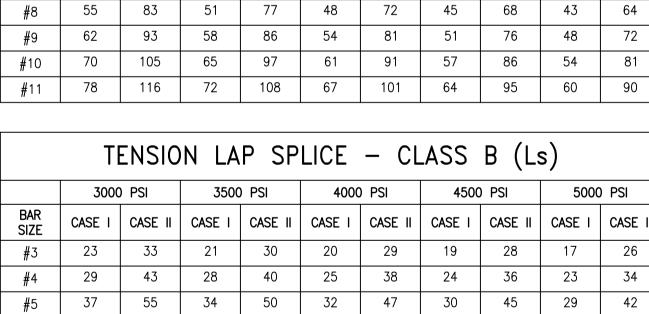
#5 @ 18"

S300  $\int SCALE$ : 1 1/2" = 1'-0"

**TENSION** 

LAP SPLICE

"
#4
#5
#6
#7
#8
#9
#10
#11



38

55

63

80

88

56

82

94

106

119

132

36

52

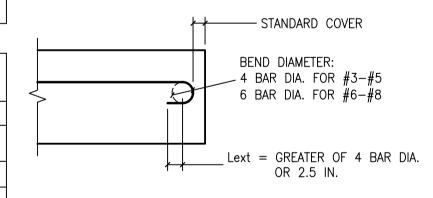
59

75

84

BEND DIAMETER: 6 BAR DIA. FOR #3-#8 8 BAR DIA. FOR #9-#11	
Lext = 12xBAR DIA. (MIN.)	
STANDARD COVER	

STANDARD HOOK (90 DEGREE)



STANDARD HOOK (180 DEGREE)

(Ls)	(Ld)				
<u>CORNER</u>	<u>CORNER</u>				
STANDARD COVER, TYP.  TWO VERTICAL BARS AT WALL END. SIZE OF BAR TO MATCH THE LARGER OF TYPICAL VERTICAL WALL BAR REINFORCING  BAR SIZE TO MATCH LARGER OF HORIZONTAL WALL REINFORCING	TWO VERTICAL BARS AT WALL END. SIZE OF BAR TO MATCH TYPICAL VERTICAL WALL BAR REINFORCING  180 DEG. HOOK (ALT. SIDES). INCLINE HOOK TO ACHIEVE APPLICABLE BAR CLEARANCE REQUIREMENTS  BAR SIZE TO MATCH HORIZONTAL WALL REINFORCING				
<u>FREE END OF WALL</u>	<u>FREE END OF WALL</u>				

MATCH HORIZ.

BAR SIZE AND

SPACING, U.N.O.

DEVELOPMENT

LENGTH

TWO LAYERS OF REINFORCING BARS (PLAN)

SINGLE LAYER OF REINFORCING BARS (PLAN)

MATCH HORIZ.

BAR SIZE AND

SPACING, U.N.O.

STANDARD DETAIL: HORIZONTAL REINFORCEMENT AT CONCRETE WALL CORNERS, INTERSECTIONS, AND ENDS S300 / SCALE: N.T.S.

1. VALUES PROVIDED IN THE TENSION DEVELOPMENT LENGTH AND TENSION LAP SPLICE TABLES CORRESPOND TO NORMAL WEIGHT CONCRETE AND UNCOATED BARS. TABLES ARE NOT APPLICABLE FOR HOOKED BARS, HEADED BARS, OR MECHANICALLY ANCHORED BARS.

THE FOLLOWING CASES SHALL BE CONSIDERED IN THE DETERMINATION OF REQUIRED DEVELOPMENT LENGTH AND TENSION LAP SPLICES: - CASE I:

- CLEAR SPACING AND CLEAR COVER OF BARS OR WIRES BEING DEVELOPED OR LAP SPLICED IS NOT LESS THAN THE BAR DIAMETER, AND STIRRUPS OR TIES THROUGHOUT Ld NOT LESS THAN CODE MINIMUM; OR

54 34

77 | 50

89 | 56

112 71

124 78

63

99

- CLEAR SPACING OF BARS OR WIRES BEING DEVELOPED OR LAP SLICED NOT LESS THAN 2x BAR DIAMETER, AND CLEAR COVER NOT LESS THAN THE BAR DIAMETER. - CASE II: ALL OTHER CONDITIONS

4. FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABLE VALUES BY 1.33.

5. FOR EPOXY COATED BARS WITH: - CLEAR COVER LESS THAN 3 BAR DIAMETER OR CLEAR SPACING LESS THAN 6 BAR DIAMETER, MULTIPLY TABLE VALUES BY 1.5.

- ALL OTHER CONDITIONS, MULTIPLY TABLE VALUES BY 1.2. 6. IF MORE THAN 12 IN. OF FRESH CONCRETE IS PLACED BELOW HORIZONTAL REINFORCEMENT, MULTIPLY TABLE VALUES BY 1.3. THIS CONDITION INCLUDES, BUT IS NOT LIMITED TO, TOP BARS IN SLABS, FOOTINGS AND BEAMS THAT ARE GREATER THAN 13" THICK, AND ALL HORIZONTAL WALL REINFORCING.

ND

S300 SCALE: N.T.S.

STANDARD	) DI	ETAIL:	TENSION	DEVE	LOP	MENT	LENGTH	A١
SPLICING	OF	STEEL	REINFOR	CING	IN	CONC	RETE	

S300

#### **HISTORIC AREA WORK PERMIT**

Address: 20 W Kirke St., Chevy Chase MD

Applicant: Elizabeth Williams Applicant's Agent: Avantika Dalal

Date: 03/18/2025

HAWP #1109308

#### **Written Narrative**

#### Alteration

The proposed project includes restoration/ reconstruction of side porch and interior renovations to (2) second floor bedroom suites, including bathrooms.

#### Landscaping

Addition of stone patio and 18" height seat wall entirely on owner's property as shown on Site Plan.

#### Tree Removal

On the East side of the house, an existing tree separates the proposed outdoor areas. The project proposes to remove the tree to unify the outdoor spaces.